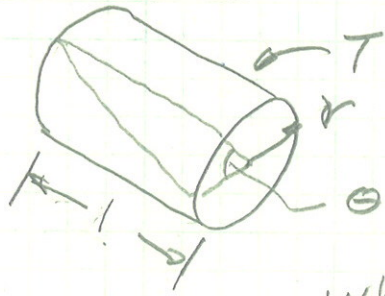


TORSION CIRCULAR SECTION 5



The shear at any point in the cylinder can be computed with,

$$\tau = \frac{Tp}{J}$$

where

T = the torque

p = the distance from the center

J = the polar area moment of inertia

The maximum shear

$$\tau_{max} = \frac{Tr}{J}$$

← RADIUS of cylinder

The angular deflection is

$$\theta = \frac{Tz}{JG}$$

$J = \frac{\pi d^4}{32}$ solid

$J = \frac{\pi (d_o^4 - d_i^4)}{32}$ Hollow

where

T = Torque

z = Length of the cylinder

J = polar area moment of inertia

G = The shear modulus

$$G = \frac{E}{2(1+\nu)}$$

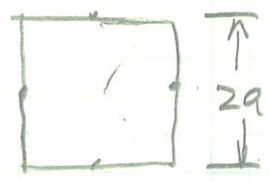
← young's Modulus
← poisson's ratio

NON-CIRCULAR SECTIONS

$$\tau_{max} = \frac{T}{Q} \quad \text{--- determined by geometry}$$

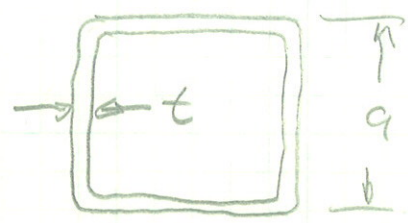
$$\theta = \frac{TL}{KG} \quad \text{--- determined by geometry}$$

SOLID SQUARE



$$K = 2,25 a^4 \quad Q = \frac{a^3}{0.6}$$

HOLLOW SQUARE



$$K = \frac{2t^2(a-t)^4}{2at - 2t^2}$$

$$Q = 2t(a-t)^2$$

EXAMPLE

Given a square hollow tube

Steel $a = 25 \text{ mm}$ $t = 1 \text{ mm}$ $G = 80.8 \text{ GPa}$
 Length = 1 meter $T = 10 \text{ N-m}$

The tube sizes are expressed in mm but we would like them in meters

$$a = 25 \text{ mm} = .025 \text{ m}$$

$$t = 1 \text{ mm} = .001 \text{ m}$$

$$K = \frac{2t^2(a-t)^4}{2ab - 2t^2} = \frac{2(.001)(.025-.001)^4}{2(.025)(.001) - 2 \cdot .001^2}$$

$$K = 1.382 \text{ E-}8 \text{ m}^4$$

COMPUTING THE AMOUNT OF TWIST

$$\theta = \frac{T L}{G K} = \frac{10(1)}{(8.08 \text{ E}10)(1.382 \text{ E-}8)} = 0.00895 \text{ rad}$$

CONVERTING TO DEGREES

$$\theta = 0.51^\circ$$

COMPUTING THE SHEAR

$$Q = 2t(a-t)^2 = 2(.001)(.025-.001)^2$$

$$Q = 1.152 \text{ E-}6 \text{ m}^3$$

$$\tau = \frac{T}{Q} = \frac{10}{1.152 \text{ E-}6} = 8.7 \text{ MPa}$$

$$\text{or } 1259 \text{ psi}$$

TRANSVERSE SHEAR

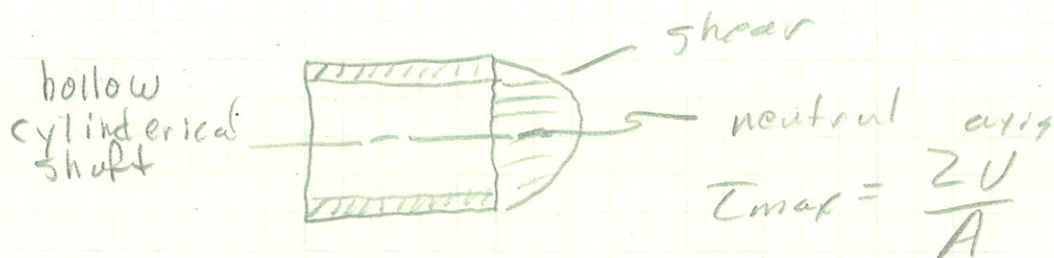
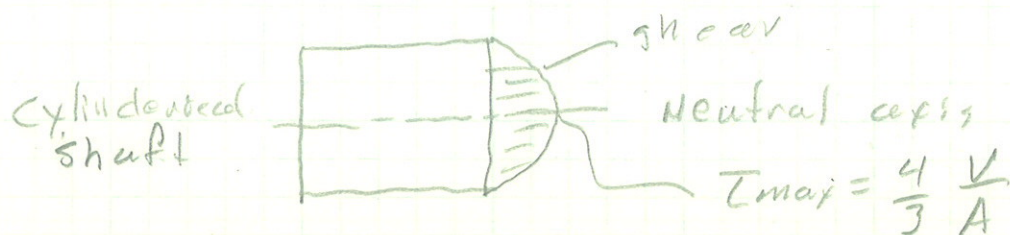
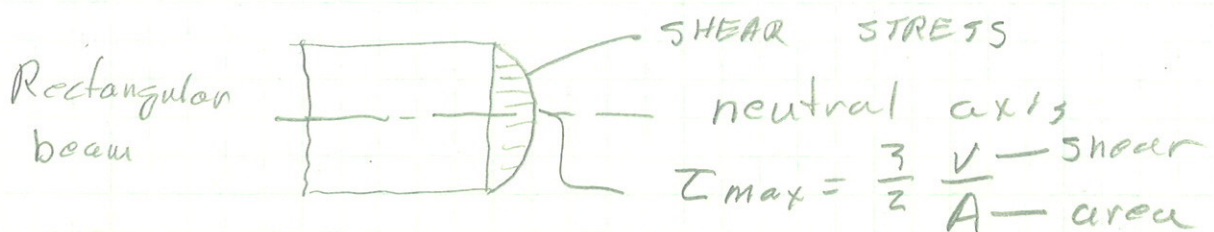
In a previous section, we looked at bending moments in beams. Loads on beams produce moments and shear forces. If the beam is long the stresses caused by the moment $\frac{M c}{I}$ dominate

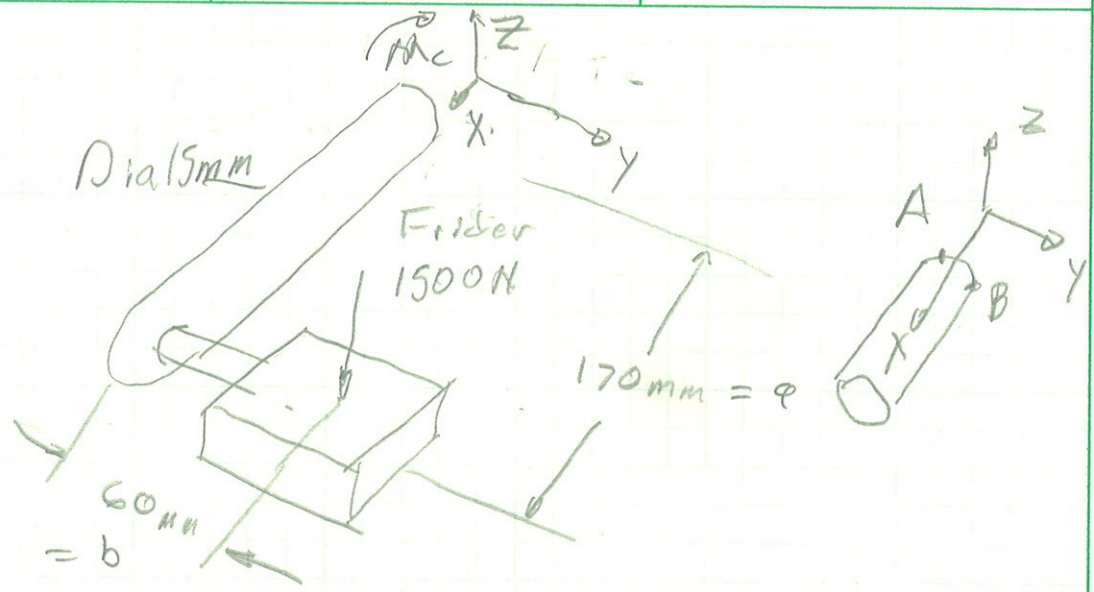
and we can exclude the shear forces in our calculations of the principal stresses,

If the beam is relatively short
Length $< 10 \cdot$ Height

Then the shear should be considered
SHEAR PROFILE

The shear profile is not constant throughout the beam. It varies, usually reaching maximum somewhere the neutral axis. The actual value depends upon the shape of the beam.



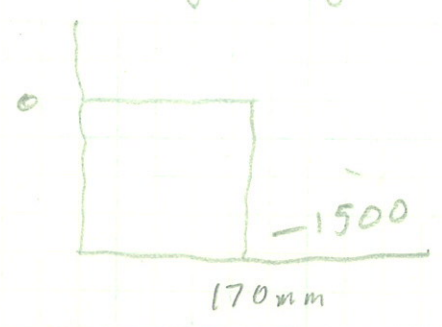


$$M = F_{\text{rider}} \cdot a = (1500)(.170) = 255 \text{ Nm}$$

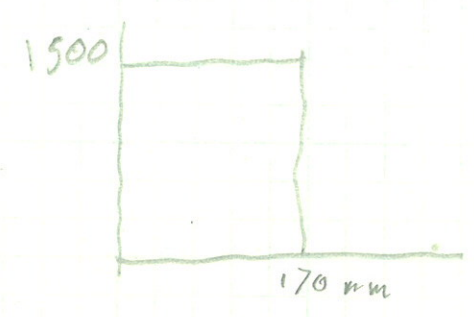
$$T = F_{\text{rider}} \cdot b = (1500)(.06) = 90 \text{ Nm}$$

Let $F = F_{\text{rider}} = 1500 \text{ N}$

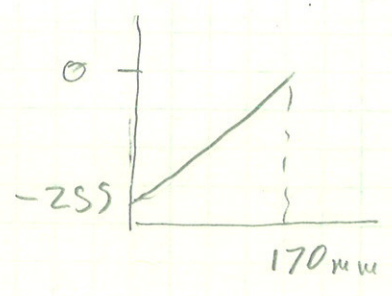
Loadings Diagram



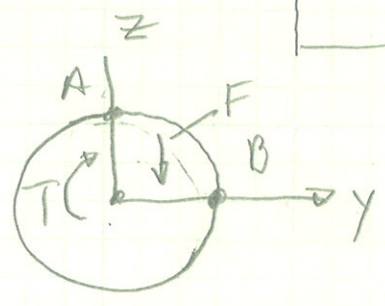
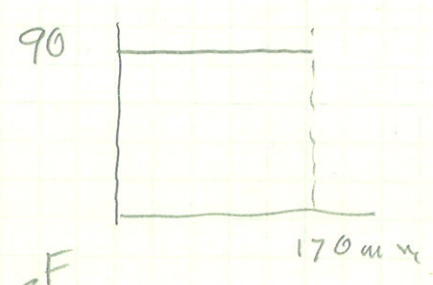
SHEAR DIAGRAM

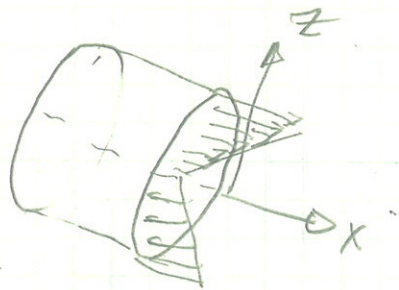


Moment Diagram



Torque Diagram





BENDING NORMAL STRESSES

$$\sigma_x = \frac{M c}{I} \quad I = \frac{\pi d^4}{64}$$



TORSION SHEAR

$$\tau_{xy} = \frac{T r}{J} \quad J = \frac{\pi d^4}{32}$$



TRANSVERSE SHEAR

$$\tau_{max} = \frac{4 V}{3 A}$$

AT POINT A

$$I_a = \frac{\pi d^4}{64} = \frac{\pi (.015)^4}{64} = 2.485 \times 10^{-9} \text{ m}^4$$

Bending stress X direction

$$\sigma_x = \frac{M c}{I} = \frac{255 (.0075)}{2.485 \times 10^{-9}} = 770 \text{ MPa}$$

$$\sigma_y = 0$$

Shear stress due to torque

$$\tau_{xy} = \frac{T r}{J} = \frac{90 (.0075)}{2 (2.485 \times 10^{-9})} = 136 \text{ MPa}$$

Principal stress at A

$$\sigma_1 = \frac{\sigma_x + \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_1 = \frac{770+0}{2} + \sqrt{\left(\frac{770-0}{2}\right)^2 + 136^2}$$

$$\underline{\sigma_1 = 793 \text{ MPa}}$$

$$\underline{\sigma_2 = 0 \text{ MPa}}$$

$$\sigma_3 = \frac{\sigma_x + \sigma_y}{2} - \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_3 = \frac{770}{2} - \sqrt{\left(\frac{770}{2}\right)^2 + 136^2}$$

$$\underline{\sigma_3 = -23 \text{ MPa}}$$

Principal Stress at B

No bending stresses at B since it is on the neutral axis

Transverse Shear

$$\tau_{xz} = \frac{4}{3} \frac{F}{A} \rightarrow \text{Transverse shear}$$

$$A = \frac{\pi d^2}{4} = \frac{\pi (0.015)^2}{4} = 1.77 \text{E-4 m}^2$$

$$\tau_{xz} = \frac{4 (1500)}{3 (1.77 \text{E-4})} = 11.3 \text{EG}$$

Torsion shear

$$\tau_{xz} = 136 \text{EG}$$

Total shear = Transverse + Torsion

$$\tau_{xz} = 11.3 \text{ EG} + 136 \text{ EG} = 147.3 \text{ EG}$$

Normal stresses

$$\sigma_x = 0 \quad \sigma_z = 0$$

$$\sigma_1 = \frac{\sigma_x + \sigma_z}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_z}{2}\right)^2 + \tau_{xz}^2} = 147.3 \text{ MPa}$$

$$\sigma_2 = 0$$

$$\sigma_3 = \frac{\sigma_x + \sigma_z}{2} - \sqrt{\left(\frac{\sigma_x - \sigma_z}{2}\right)^2 + \tau_{xz}^2} = -147.3 \text{ MPa}$$

STRESS ON PEDAL SCREW

$z = 1500\text{ N}$



$$M = 1500 \cdot (.060) = 90\text{ N}\cdot\text{m}$$



DISTANCE TO NEUTRAL AXIS

$$c = \frac{d}{2} = \frac{.012}{2} = .006\text{ m}$$

MOMENT OF INERTIA

$$I = \frac{\pi d^4}{64} = \frac{\pi (.012)^4}{64} = 1.0184 \times 10^{-9}\text{ m}^4$$

Bending Stress Y Direction

$$\sigma_y = \frac{MC}{I} = \frac{90(.006)}{1.0184 \times 10^{-9}} = 530.5\text{ MPa}$$

$$\sigma_z = 0\text{ MPa}$$

$$\tau_{xy} = 0$$

since these are zero

$$\sigma_1 = \sigma_y$$